1.1 BACKFILL SOILS / DRAINAGE STONE

1.1.1 REINFORCED BACKFILL MATERIAL SPECIFIED BELOW SHALL BE FREE DRAINING. REINFORCED BACKFILL MATERIALS SHALL BE APPROVED BY THE OWNER OR OWNER'S REPRESENTATIVE AND SHALL MEET THE PHYSICAL PROPERTY REQUIREMENTS DEFINED IN SECTION 6.0. THE REINFORCED BACKFILL MATERIAL SHALL BE CRUSHED ANGULAR STONE MEETING THE FOLLOWING GRADATION:

SIEVE SIZE	PERCENT PASSIN	
2 inch	100	
1 inch	30-100	
3/4 inch	10-70	
1/2 inch	0-40	
No. 4	0-10	
Company of		

1.1.2 ON-SITE FILL

ON-SITE FILL MATERIAL SHALL BE ON-SITE OR IMPORTED COMPRESSIBLE SOIL CLASSIFIED PER THE UNIFIED SOIL CLASSIFICATION SYSTEM AS LOW PLASTICITY (MAX PI=25), COMPACTED TO 95% STD. PROCTOR DENSITY.

HAVE A LIQUID LIMIT OF LESS THAN 40 AND A PLASTICITY INDEX OF LESS THAN 20. REINFORCED BACKFILL MATERIAL SHALL BE CLASSIFIED PER THE UNIFIED SOIL CLASSIFICATION SYSTEM AS LOW PLASTICITY OR NON-PLASTIC SOILS.

1.2 THE PORTION OF THE REINFORCED BACKFILL MATERIAL PASSING THE No. 40 SIEVE SHALL

1.3 GEOGRID REINFORCING SHALL BE TENSAR UX1400 UNIAXIAL GEOGRID AS MANUFACTURED BY THE TENSAR CORPORATION. DESIGNS PRESENTED HEREIN ARE VALID FOR TENSAR GEOGRIDS OR ENGINEER APPROVED EQUAL.

1.4 WALL FACING SHALL BE CHOPPED LIMESTONE BLOCK.

1.5 GEOTEXTILE FABRIC SHALL BE MIRAFI 140N OR APPROVED EQUAL.

1.6 REINFORCING BARS SHALL BE ASTM A615, GRADE 60.

1.7 MORTAR SHALL BE IN ACCORDANCE WITH THE CITY OF AUSTIN SPEC. 403S.

1.8 CONSTRUCTION ADHESIVE SHALL BE SHEP POXY AS MANUFACTURED BY SHEPLER'S PERFORMANCE PRODUCTS OR APPROVED EQUAL.

2.0 TECHNICAL REQUIREMENTS

PRIOR TO CONSTRUCTION OF THE GEOGRID REINFORCED WALL, THE CONTRACTOR SHALL CLEAR AND GRUB THE REINFORCED BACKFILL ZONE, REMOVING TOPSOILS, BRUSH, SOD OR OTHER ORGANIC OR DELETERIOUS MATERIALS. ANY UNSUITABLE SOILS SHALL BE OVER-EXCAVATED, REPLACED AND COMPACTED WITH REINFORCED BACKFILL MATERIAL TO PROJECT SPECIFICATIONS OR AS OTHERWISE DIRECTED BY THE OWNER'S GEOTECHNICAL

BACKFILL MATERIALS SHALL BE PLACED FROM THE BACK OF THE BLOCK FACING UNITS TOWARDS THE TAIL OF THE GEOGRID TO ENSURE FURTHER TENSIONING.

2.3 REINFORCED BACKFILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT EXCEEDING 8 INCHES IN UNCOMPACTED THICKNESS.

2.4 ONLY HAND-OPERATED EQUIPMENT SHALL BE ALLOWED WITHIN THREE FEET OF THE BACK FACE OF WALL. COMPACTION SHALL BE ACHIEVED BY A LIGHTWEIGHT MECHANICAL TAMPER, ROLLER OR VIBRATORY SYSTEM. CARE SHALL BE EXERCISED DURING THE COMPACTION PROCESS TO AVOID MISALIGNMENT OF THE BLOCK UNITS.

2.5 REINFORCED BACKFILL MATERIAL DOES NOT REQUIRE DENSITY TESTING. COMPACTION FOR THIS TYPE OF MATERIAL SHALL CONTINUE UNTIL THERE IS NO EVIDENCE OF FURTHER COMPACTION, OR AS DIRECTED BY THE OWNER'S GEOTECHNICAL ENGINEER. SHOULD THE SUBGRADE, FOR ANY REASON OR CAUSE, LOSE THE REQUIRED STABILITY OR FINISH, IT SHALL BE RECOMPACTED AND REFINISHED AT THE CONTRACTOR'S EXPENSE.

THE CONTRACTOR SHALL HAVE AN APPROVED SET OF CONSTRUCTION DRAWINGS AND CONTRACT SPECIFICATIONS ON-SITE AT ALL TIMES DURING CONSTRUCTION OF THE RETAINING 3.0 GEOGRID PLACEMENT

3.1 GEOGRID SHALL BE PLACED AT THE LOCATIONS AND ELEVATIONS SHOWN ON THE CONSTRUCTION DRAWINGS.

3.2 GEOGRID EMBEDMENT LENGTH (GEL) SHALL BE AS SHOWN ON THE CONSTRUCTION DRAWINGS. REINFORCED BACKFILL ZONE LENGTH IS MEASURED FROM THE FRONT FACE

3.3 GEOGRID REINFORCEMENT SHALL BE CONTINUOUS THROUGHOUT THE DESIGNATED EMBEDMENT LENGTH(S).

3.4 THE CONNECTION OF THE GEOGRID TO THE BLOCK SHALL BE A POSITIVE-MECHANICAL

TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID. A MINIMUM FILL THICKNESS OF SIX INCHES IS REQUIRED FOR OPERATION OF TRACKED VEHICLES OVER THE GEOGRID. TURNING OF TRACKED VEHICLES SHOULD BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND/OR THE

3.6 RUBBER-TIRED VEHICLES MAY PASS OVER THE GEOGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE

3.7 UNIAXIAL GEOGRID SHALL BE ROLLED OUT WITH THE LONG AXIS OF THE APERTURES (MACHINE DIRECTION) PERPENDICULAR TO THE WALL FACE.

3.8 UNIAXIAL GEOGRIDS SHALL BE CUT NEXT TO THE CROSS-MACHINE DIRECTION BAR. THE CROSS-MACHINE DIRECTION BAR SHALL BE PLACED AND PULLED TAUT PRIOR TO FILL

3.9 A MINIMUM OF 3 INCHES OF FILL MATERIAL SHALL BE REQUIRED BETWEEN LAYERS OF

3.10 NO CHANGES TO THE GEOGRID LAYOUT INCLUDING, BUT NOT LIMITED TO LENGTH,

GEOGRID TYPE OR ELEVATION SHALL BE MADE WITHOUT THE EXPRESSED PRIOR WRITTEN

4.0 BLOCK PLACEMENT

CONSENT OF GEOSOLUTIONS INC.

4.1 THE ALLOWABLE HORIZONTAL AND VERTICAL TOLERANCE FOR THE ERECTION OF THE WALLS SHALL BE LIMITED TO 1.5 inch IN 10.0 FEET OF LENGTH OR HEIGHT.

5.0 DRAINAGE

5.1 FOR WALLS NOT INCORPORATING FREE-DRAINING CRUSHED STONE BACKFILL, THE BACKFILL SURFACE SHALL BE GRADED AWAY FROM THE WALL FACE A MINIMUM OF 2 PERCENT SLOPE AND A TEMPORARY SOIL BERM SHALL BE CONSTRUCTED NEAR THE WALL CREST TO PREVENT SURFACE WATER RUNOFF FROM OVERTOPPING THE WALL. GRADING SHALL BE PERFORMED AT THE END OF EACH WORK DAY.

5.2 AT THE END OF EACH WORKDAY, BACKFILL SURFACE SHALL BE COMPACTED WITH A SMOOTH WHEEL ROLLER TO MINIMIZE PONDING OF WATER AND SATURATION OF THE

5.3 PERMANENT SURFACE WATER DIVERSION AND/OR COLLECTION SHALL BE AS REQUIRED AND PROVIDED BY THE OWNER OR OWNER'S REPRESENTATIVE.

5.4 THE RETAINING WALL HAS BEEN DESIGNED ON THE ASSUMPTION THAT THE REINFORCED BACKFILL MATERIAL SHALL BE FREE OF SUBSURFACE DRAINAGE OF WATER (SEEPAGE). IF GROUND WATER IS ENCOUNTERED, GEOSOLUTIONS INC. SHALL BE CONTACTED

5.5 CARE SHALL BE TAKEN NOT TO CONTAMINATE THE GEOTEXTILE FABRIC AND/OR DRAINAGE STONE WITH FINE-GRAINED SOILS OR OTHER DELETERIOUS MATERIALS.

6.0 DESIGN PARAMETERS

6.1 DESIGN OF THE RETAINING WALLS IS BASED ON THE FOLLOWING PARAMETERS:

COHESION	WT
0 psf	125 p
0 psi	125 p
	0 psf

6.2 FACTORS OF SAFETY:

7.0 SPECIAL PROVISIONS

6.2.1 INTERNAL STABILITY: DESIGN MINIMUM FACTOR OF SAFETY FOR UNCERTAINTIES = 1.5 = 1.5 MINIMUM FACTOR OF SAFETY FOR GEOGRID PULLOUT MINIMUM FACTOR OF SAFETY FOR SLIDING AT LOWEST GEOGRID = 1.5

SOIL-GEOGRID INTERACTION COEFFICIENT PERCENT COVERAGE OF GEOGRID 6.2.2 EXTERNAL STABILITY:

= 1.5 = 1.5

= 2.0 = 2.0MINIMUM FACTOR OF SAFETY FOR OVERTURNING 6.3 SURCHARGE LOADING = 250 psf

MINIMUM FACTOR OF SAFETY FOR SLIDING AT BASE

7.1 THE DESIGN PRESENTED HEREIN IS BASED ON SOIL PARAMETERS, FOUNDATION CONDITIONS, GROUNDWATER CONDITIONS, AND LOADINGS STATED IN SECTION 6.0.

7.2 LOCATIONS AND GEOMETRY OF EXISTING STRUCTURES AND GRADE ABOVE AND BELOW THE WALLS MUST BE VERIFIED BY THE OWNER OR OWNER'S REPRESENTATIVE PRIOR TO

7.3 THE OWNER OR OWNER'S REPRESENTATIVE IS RESPONSIBLE FOR REVIEWING AND VERIFYING THAT THE ACTUAL SITE CONDITIONS ARE AS DESCRIBED IN SECTION 6.0 PRIOR TO AND DURING CONSTRUCTION. THE OWNER OR OWNER'S REPRESENTATIVE SHALL BE ON-SITE TO ASSURE THE PROVISIONS IN THE CONSTRUCTION NOTES ARE

7.4 THE SOIL DESIGN PARAMETERS STATED IN SECTION 6.0 ARE THE RESPONSIBILITY OF GEOSOLUTIONS, INC. THE OWNER OR OWNER'S REPESENTATIVE SHALL CONTACT

GEOSOLUTIONS INC. IF THE SOILS ENCOUNTERED APPEAR TO VARY FROM THOSE ENCOUNTERED AT THE BEGINNING OF CONSTRUCTION. 7.5 IF ANY ROCK FORMATIONS AND/OR GROUNDWATER ARE ENCOUNTERED DURING THE

7.6 ANY REVISIONS TO DESIGN PARAMETERS STATED IN SECTION 6.0 OR STRUCTURE

CONSTRUCTION OF THIS WALL, IMMEDIATELY CONTACT THE OWNER OR OWNER'S

GEOMETRY SHALL REQUIRE DESIGN MODIFICATIONS PRIOR TO PROCEEDING WITH

7.7 THIS DESIGN IS VALID ONLY FOR THE SONGHAI AT WESTGATE PROJECT, AUSTIN, TEXAS.

8.0 OWNER'S RESPONSIBILITIES

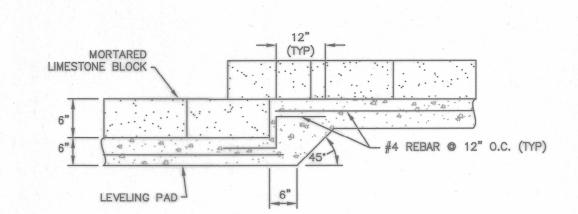
OWNER SHALL BE RESPONSIBLE FOR CONFIRMING THAT ALL REQUIREMENTS SET FORTH ON THESE DRAWINGS ARE MET. ASSIGNMENT OR DELEGATION OF RESPONSIBILITIES BY OWNER TO OWNER'S REPRESENTATIVE SHALL NOT RELIEVE OWNER OF RESPONSIBILITY OF CONFIRMING THAT ALL REQUIREMENTS SET FORTH HEREIN ARE MET.

8.2 OWNER (OR OWNER-DESIGNATED REPRESENTATIVES) RESPONSIBILITIES, AS DESCRIBED IN PREVIOUS SECTIONS OF THESE NOTES, SHALL INCLUDE:

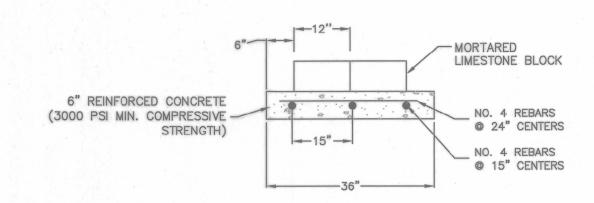
8.2.1 PERMANENT SURFACE WATER DIVERSION (SECTION 5.0).

8.2.2 CONFIRMATION OF GEOMETRY AND LOADING CONDITIONS FOR AREAS ADJACENT TO WALL

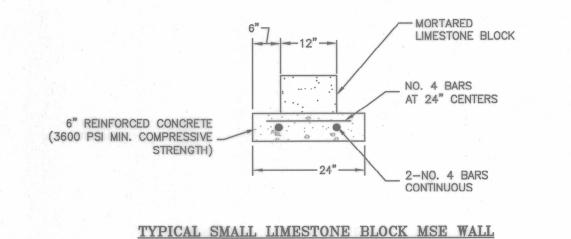
8.2.3 ASSURING CONFORMITY WITH CONSTRUCTION DRAWINGS AND NOTES DURING CONSTRUCTION BY ON-SITE INSPECTION (SECTION 7.0).



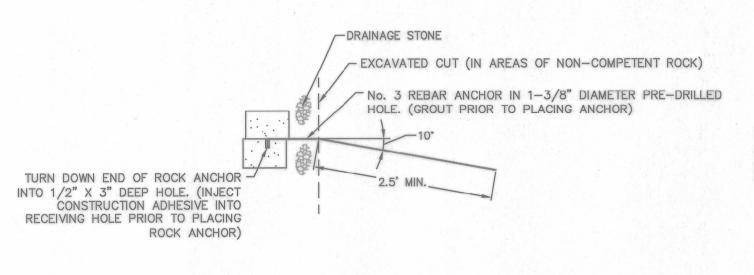
TYPICAL SMALL LIMESTONE BLOCK MSE WALL LEVELING PAD STEP DETAIL N.T.S.



GRAVITY WALL LEVELING PAD DETAIL - 2 BLOCKS



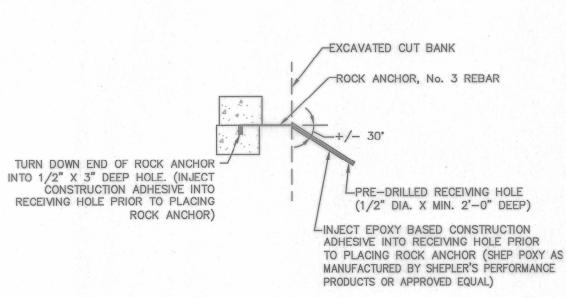




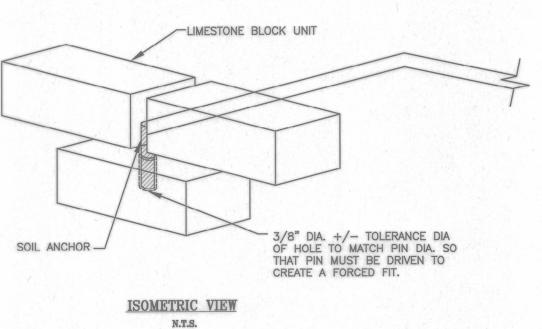
NOTE: GROUT SHALL CONSIST OF A PUMPABLE MIXTURE OF TYPE I, II OR III PORTLAND CEMENT, SAND, WATER AND ADMIXTURES. GROUT SHALL MEET ASTM C-150 WITH 3500 PSI, 7 DAY COMPRESSIVE STRENGTH.

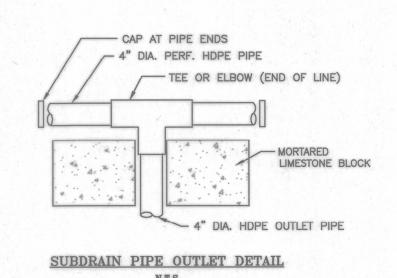
LIMESTONE BLOCK TO NON-COMPETENT ROCK CUT CONNECTION DETAIL

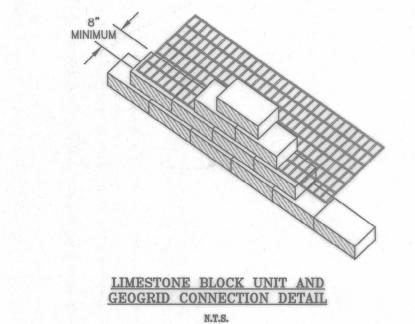
THIS DETAIL TO BE USED IN AREAS OF NON-COMPETENT ROCK, ONLY WITH PRIOR APPROVAL OF DESIGN ENGINEER AFTER VISUAL INSPECTION OF AREA IN QUESTION.

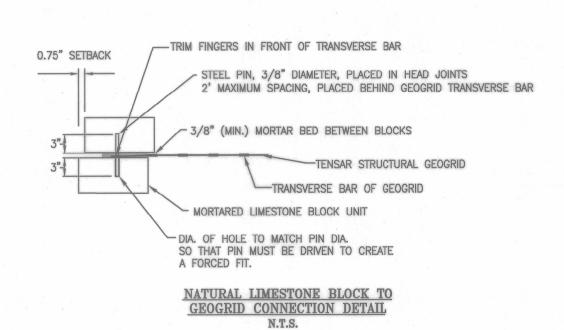


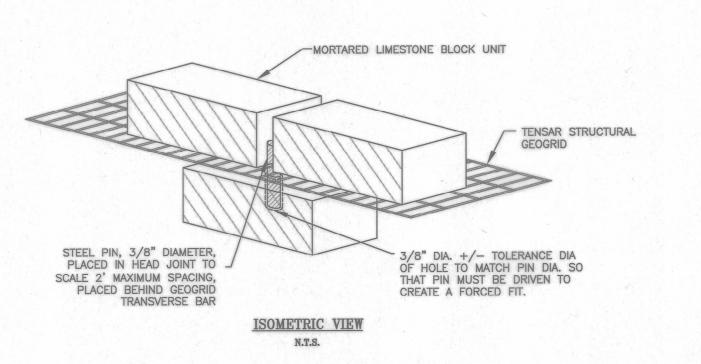
LIMESTONE BLOCK TO CUT BANK CONNECTION DETAIL (ROCK ANCHOR) N.T.S.

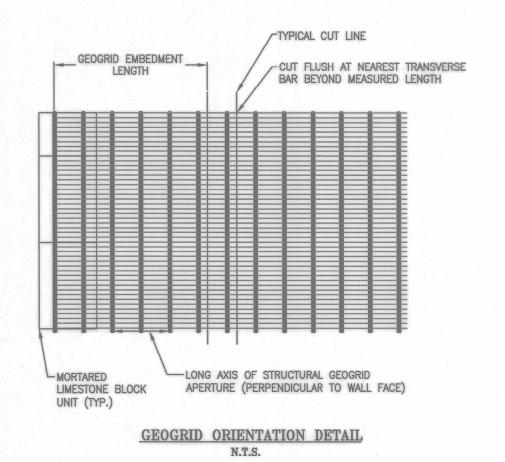


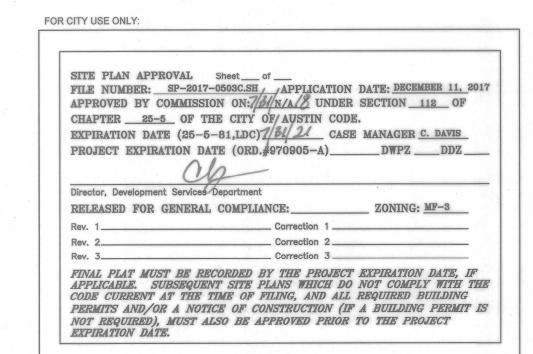












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